

BUILDING DIVISION DIRECTIVE B-004

INTERIM DIRECTIVE ON MANUFACTURED PANELS AND BRACE FRAMES BY HARDY FRAMES

Background

ICC-ES published the acceptance criteria for cold-formed steel shear panels (AC 322-07) last October 2007. However, shear panels and brace frames by Hardy Frames are not approved by ICC-ES as alternatives to light-framed shear walls described in the 2006 International Building Code (IBC) and 2007 California Building Code (CBC). The City of San Jose Building Division engineers have reviewed the most recent test data under the recently adopted acceptance criteria (AC 322-07) to determine the allowable load tables of Hardy Frame panels and brace frames for use in structures designed under the 2007 CBC.

Guidelines for Design, Plan Check and Field Inspection

Until evaluated using recently published criteria and approved for use under the 2006 IBC by ICC-ES, Hardy Frame panels and brace frames may be used as alternatives to light-framed shear walls in the City of San Jose only when the following criteria are met:

- 1. Only the "X Series" models listed on attached Table 1-SJ, Table 2-SJ, and Table 3-SJ are allowed to be used in light-framed construction.
- 2. Hardy *brace frames* shall be installed directly on concrete foundation without mud sill, raised floor framing, or cripple walls.
- 3. Hardy *panels* are allowed to be installed on concrete foundation without mud sill or on raised floor framing. When used on raised floor framing, Installation of Hardy panels shall be in accordance with all the requirements listed below:
 - Raised floor framing under Hardy panel shall have a single 4x or 6x rim and a single 3x sill plate sitting directly on concrete foundation;
 - The 4x or 6x rim shall be of Structural Composite Lumber (SCL) with the moisture content of not more than 16% at the time of installation;
 - Both 4x or 6x rim and 3x sill plate shall continue at least 12" beyond each side of the Hardy panel except around the outside corner of a building, where Hardy panels shall be installed a minimum of 3" away from the end of the wall;
 - No cripple wall is allowed under Hardy panels.
- 4. For 1-story conventional light-framed construction, Hardy panels and brace frames may be allowed as alternative bracing panels sitting on concrete foundation or raised floor framing, if the panel or brace frame is 18" or wider.

- 5. Balloon framed Hardy Frame panels or brace frames are not allowed regardless of the foundation system used.
- 6. Double panels or brace frames (back-to-back) are not allowed at any location or along any braced wall line.
- 7. Max. 8" tall filler may be allowed between double top plates and Hardy panels/brace frames if appropriate attachment details are designed by the engineer of record and approved by the City. When the filler is thicker than 1.5", the allowable values of the next panel height shall be used for the design. The filler/header installed on top of Hardy panels/brace frames shall be of Structural Composite Lumber (SCL) with a moisture content of not more than 16% at the time it is fastened to the panels.
- 8. The allowable loads of Hardy Frames panels and brace frames shall be determined from the attached Table 1-SJ, Table 2-SJ, and Table 3-SJ. The allowable shear capacity shall be reduced in proportion to the actual tension capacity of hold-down bolts based on reduced edge and/or end distances and the actual strength of the concrete foundation.
- 9. Neither Hardy Frame panels nor brace frames shall be used for the design of buildings of occupancy category III and IV per Table 1604.5 of the 2007 CBC.
- 10. R=6.5 may be used for the design of manufactured Hardy Frame panels and brace frames in the direction considered. When Hardy brace frames/panels are used in line with other types of shear panels, only one type of panels shall be considered as the lateral resistance element along such wall line. The design shall be governed by whichever has the lowest allowable shear capacity and "R" value.
- 11. Except for one-story conventional construction, the Engineer of Record shall check:
 - Concrete foundation/grade beam capacity and soil bearing pressure under coupled tension and compression load at ends of Hardy Frame panels or brace frames.
 - Tension capacity of hold-down bolts based on reduced edge and end distance as detailed on the plans.
- 12. All installations shall use respective manufacturer's approved templates, and follow the installation details and instructions listed on sheets HF1-SJ, HF2-SJ, and HF3-SJ.
- 13. This directive will be effective starting May 21, 2008, and subject to reevaluation after December 31st, 2008.

Approved by

Edward Tolentino

Chief Building Official

			V - Seismic Governs			V - Wi	nd Gove	rns		Add'l ⁸	Add'l ⁸
			Allowable ^{2, 3}			Allowable ²				Vertical	Vertical
	Net	HD Bolt	In-Plane	Drift 5	Uplift 4, 5	In-Plane	Drift 5	Uplift 4,5	Screw	Load	Load
Model	Height	Dia.	Shear V	at V	at V	Shear V	at V	at V	Qty ^{6, 7}	P _{seismic}	P_{wind}
Number	H (in)	(in)	(lbs)	(in)	(lbs)	(lbs)	(in)	(lbs)	at Top	(lbs)	(lbs)
12" Wide Panels										(1.0.5)	
HF X 12x78-1 1/8	78"	1 1/8	1,277	1,284	0.20	9,775	4	4,000	4,000		
HF X 12x8-1 1/8	92-1/4"	1 1/8	1,079	0.27	9,708	1,086	0.28	9,775	4	4,000	4,000
HF X 12x9-1 1/8	104-1/4"	1 1/8	956	0.35	9,726	961	0.35	9,775	4	2,000	2,000
HF X 12x10-1 1/8	116-1/4"	1 1/8	859	0.43	9,739	862	0.43	9,775	4	1,000	1,000
	18" Wide Panels										
HF X 18x78-1 1/8	78"	1 1/8	2,945	0.19	14,137	2,938	0.19	14,137	9	4,000	4,000
HF X 18x8-1 1/8	92-1/4"	1 1/8	2,487	0.26	14,119	2,487	0.26	14,119	8	4,000	4,000
HF X 18x9-1 1/8	104-1/4"	1 1/8	2,202	0.33	14,130	2,202	0.33	14,130	7	4,000	4,000
HF X 18x10-1 1/8	116-1/4"	1 1/8	1,975	0.41	14,126	1,975	0.41	14,126	6	4,000	4,000
HF X 18x11-1 1/8	128-1/4"	1 1/8	1,790	0.50	14,128	1,836	0.50	14,128	6	4,000	4,000
HF X 18x12-1 1/8	140-1/4"	1 1/8	1,641	0.60	14,162	1,641	0.60	14,162	5	4,000	4,000
HF X 18x13-1 1/8	152-1/4"	1 1/8	1,508	0.71	14,129	1,508	0.71	14,129	5	4,000	4,000
					24" Wide P	anels			-		
HF X 24x78-1 1/8	78"	1 1/8	4,358	0.11	15,276	4,358	0.11	15,276	13	4,000	4,000
HF X 24x8-1 1/8	92-1/4"	1 1/8	3,685	0.14	15,278	3,685	0.14	15,278	11	4,000	4,000
HF X 24x9-1 1/8	104-1/4"	1 1/8	3,261	0.18	15,278	3,261	0.18	15,278	10	4,000	4,000
HF X 24x10-1 1/8	116-1/4"	1 1/8	2,924	0.22	15,275	2,924	0.22	15,275	9	4,000	4,000
HF X 24x11-1 1/8	128-1/4"	1 1/8	2,651	0.27	15,279	2,651	0.27	15,279	8	3,000	3,000
HF X 24x12-1 1/8	140-1/4"	1 1/8	2,422	0.32	15,264	2,422	0.32	15,264	7	3,000	3,000
HF X 24x13-1 1/8	152-1/4"	1 1/8	2,232	0.38	15,276	2,232	0.38	15,276	6	1,000	1,000

For **SI**: 1 inch = 25.4 mm, 1 lbf = 4.45 N

Notes:

- 1) Installation on Concrete Foundations only.
- 2) 1.33 increase IS NOT ALLOWED for the loads shown.
- 3) For seismic loads under the 2006 IBC or 2007 CBC (California Building Code) R=6.5, $\Omega_{\rm o}$ =3 and C_d =4.
- 4) The uplift values listed are those corresponding to the allowable shear V.
- 5) For reduced shear loading the uplift and drift are proportionately reduced.
- 6) The 1/4-inch diameter by 3-inch long wood screws are Hardy Frame HFS-series, USP WS-series (ICC-ES PFC-5634) or equal.
- 7) When installing a 2-by wood filler (specific gravity of 0.5 or greater) at the top connection, the minimum screw length is 4-1/2 in.
- 8) Load P indicates maximum allowable vertical load (assuming a uniform distribution) permitted on top of Panel in addition to the vertical load produced by overturning at the allowable shear V.
- 9) Hold downs are ASTM A36 (minimum) and the 5/8 x 10 Anchor Bolt at the 24" Panel is ASTM A307 (minimum)

Design Loads in this table have been reduced by 15% to meet the requirements of the City of San Jose Building Department.

			V - Seismic Governs			V - Wi	nd Gover	ns			Add'l 10	Add'l 10
			Allowable 3, 4			Allowable 3					Vertical	Vertical
	Net	HD Bolt	In-Plane	Drift 5,7	Uplift 6,7	In-Plane	Drift 5,7	Uplift 6,7	Screw	Screw	Load	Load
Model	Height	Dia.	Shear V	at V	at V	Shear V	at V	at V	Qty ^{8, 9}	Qty ^{8, 9}	P _{seismic}	P_{wind}
Number	H (in)	(in)	(lbs)	(in)	(lbs)	(lbs)	(in)	(lbs)	at Top	at Bottom	(lbs)	(lbs)
12" Wide Panels												
HF X 12x78-1 1/8	78"	1 1/8	893	1,226	0.40	9,330	3	4	4,000	2,600		
HF X 12x8-1 1/8	92-1/4"	1 1/8	720	0.42	6,480	985	0.48	8,866	3	4	4,000	2,600
HF X 12x9-1 1/8	104-1/4"	1 1/8	592	0.46	6,021	826	0.53	8,400	3	4	2,000	1,300
HF X 12x10-1 1/8	116-1/4"	1 1/8	494	0.51	5,601	704	0.58	7,981	3	4	1,000	650
18" Wide Panels												
HF X 18x78-1 1/8	78"	1 1/8	2,062	0.28	9,896	2,962	0.33	14,137	7	9	4,000	4,000
HF X 18x8-1 1/8	92-1/4"	1 1/8	1,741	0.36	9,884	2,487	0.43	14,119	6	8	4,000	4,000
HF X 18x9-1 1/8	104-1/4"		1,542	0.43	9,890	2,202	0.51	14,130	5	7	4,000	4,000
HF X 18x10-1 1/8	116-1/4"	1 1/8	1,363	0.50	9,747	1,975	0.60	14,126	5	6	4,000	4,000
HF X 18x11-1 1/8	128-1/4"	1 1/8	1,146	0.55	9,040	1,709	0.67	13,494	5	6	4,000	3,250
HF X 18x12-1 1/8	140-1/4"		976	0.60	8,422	1,476	0.73	12,742	4	5	4,000	2,600
HF X 18x13-1 1/8	152-1/4"	1 1/8	845	0.65	7,916	1,292	0.79	12,108	4	5	4,000	2,600
					Vide Panels							
HF X 24x78-1 1/8	78"	1 1/8	3,051	0.17	10,694	4,358	0.20	15,276	10	13	4,000	3,250
HF X 24x8-1 1/8	92-1/4"	1 1/8	2,580	0.21	10,694	3,685	0.25	15,278	8	11	4,000	3,250
HF X 24x9-1 1/8	104-1/4"		2,282	0.25	10,694	3,261	0.30	15,278	7	10	4,000	3,250
HF X 24x10-1 1/8	116-1/4"	1 1/8	2,047	0.29	10,694	2,924	0.35	15,275	7	9	4,000	2,600
HF X 24x11-1 1/8	128-1/4"	1 1/8	1,856	0.34	10,697	2,651	0.41	15,279	6	8	3,000	1,950
HF X 24x12-1 1/8	140-1/4"	1 1/8	1,695	0.39	10,685	2,422	0.47	15,264	5	7	3,000	1,950
HF X 24x13-1 1/8	152-1/4"	1 1/8	1,563	0.44	10,696	2,232	0.53	15,276	5	7	1,000	650

For SI: 1 inch = 25.4 mm, 1 lbf = 4.45 N

Notes:

- 1) Raised Floor requires minimum 3-by sill plate, 4x Engineered Wood rim and Hardy Frame Bearing Plate installed below Panel.
- 2) When seismic governs and Raised Floor conditions are different than described above, shrinkage and crushing must be considered to verify that the corresponding drift is within the code limits.
- 3) 1.33 increase IS NOT ALLOWED for the loads shown.
- 4) For seismic loads under the 2006 IBC or 2007 CBC (California Building Code) R=6.5, Ω₀ =3 and C_d =4.
- 5) Drift at Allowable Shear V considers shrinkage and crushing of wood members. Shrinkage is based on a 7% change in moisture content. Compression is based on Fc of 680 psi and a compression load equal to the uplift.
- 6) The uplift values listed are those corresponding to the allowable shear V.
- 7) For reduced shear loading the uplift and drift are proportionately reduced.
- 8) The 1/4-inch diameter by 3-inch long wood screws are Hardy Frame HFS-series, USP WS-series (ICC-ES PFC-5634) or equal.
- 9) Minimum screw length is 3-inches at the top and 4-1/2 inches at the bottom connection. When installing a 2-by wood filler (specific gravity of 0.5 or greater) at the top connection, the minimum screw length is 4-1/2 inches.
- 10) Load P indicates maximum allowable vertical load (assuming a uniform distribution) permitted on top of Panel in addition to the vertical load produced by overturning at the allowable shear V.
- 11) Hold downs are ASTM A36 (minimum).

Design Loads in this table have been reduced by 15% to meet the requirements of the City of San Jose Building Department.

Table 3-SJ: 2006 IBC Hardy Frame[®] X-Series Brace Frames - on FOUNDATIONS 1, 10 5-8-2008

			V-Seismic Governs			V-Wir	d Gover	ns			
			Allowable ^{2, 3}	Drift ⁵		Allowable ²	Drift ⁵			Add'l 8,9	Add'l 8,9
	Net	HD Bolt	In-Plane	at	Uplift 4, 5	In-Plane	at	Uplift 4, 5	Screw 6,7	Vertical	Vertical
Model	Height	Dia.	Shear V	V	at V	Shear V	V	at V	Qty	Load	Load
Number	H (in)	(in)	(lbs)	(in)	(lbs)	(lbs)	(in)	(lbs)	at Top	W (plf)	P (lbs)
32" Wide Brace Frames											
HF X 32x8	92-1/4"	7/8	2,461	0.15	7,503	2,461	0.15	7,503	12	1,500	4,000
HF X 32x9	104-1/4"	7/8	2,329	0.21	8,026	2,329	0.21	8,026	11	1,500	4,000
HF X 32x10	116-1/4"	7/8	2,087	0.25	8,020	2,087	0.25	8,020	10	1,500	4,000

For SI: 1 inch = 25.4 mm, 1 lbf = 4.45 N

Notes:

- 1) Installation on Concrete Foundations only.
- 2) 1.33 increase IS NOT ALLOWED for the loads shown.
- 3) For seismic loads under the 2006 IBC or 2007 CBC (California Building Code) R=6.5, Ω_0 =3 and C_d =4.
- 4) The uplift values listed are those corresponding to the allowable shear V.
- 5) For reduced shear loading the uplift and drift are proportionately reduced.
- 6) The 1/4-inch diameter by 3-inch long wood screws are Hardy Frame HFS-series, USP WS-series (ICC-ES PFC-5634) or equal.
- 7) When installing a 2-by wood filler (specific gravity of 0.5 or greater) at the top connection, the minimum screw length is 4-1/2 in.
- 8) Loads "W" & "P" indicate the maximum allowable vertical load permitted on top of Brace Frames, in addition to the vertical load produced by overturning at the maximum Allowable Shear V.
- 9) Loads "W" & "P" can act concurrently provided that the actual total load resultant does not exceed "P".
- 10) Hold downs are ASTM A36 (minimum).

Design Loads in this table have been reduced by 30% to meet the requirements of the City of San Jose Building Department.





